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# Development of generalized Iwan model to simulate frictional contacts with variable normal loads

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#### ABSTRACT

Most friction models are originally proposed to predict restoring forces in mechanical contacts with constant normal load. In practice the contact interface kinematics may involve normal motion in addition to the tangential displacements, leading to variation of the contact normal load. This phenomenon is observed most strongly in contacts with high lateral vibration amplitudes and is known as slap. The current study establishes a general friction model to account for variation in the normal load and enables one to predict the behavior of a contact more precisely. Iwan model (1966) [5] is a suitable candidate for contact interface modeling and is able to represent the stick-micro/macro slip behavior involved in a friction contact. This physical based model is employed in the current work and its physical parameters are generalized to include the normal load variation effects. The model is characterized by a slippage distribution density function and a linear stiffness at stick state. Both these parameters, defined in presence of constant normal load in the original model, are derived considering normal load variation leading to generalization of the contact model. Conventional models with constant normal loads produce symmetric contact interface hysteresis loops, but the developed generalized Iwan model is capable of generating asymmetric hysteresis loops similar to those frequently seen in experiments. The generalized contact model is employed to simulate the measured behavior of a beam with frictional support observed in an experimental test set-up. The contact slippage distribution function is first identified in a constant normal load condition. Next in low levels of contact preloads where variation of the normal load is significant, the identified distribution function in generalized form is employed to predict the experimental observations.

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#### 1. Introduction

Contact modeling is of considerable practical importance and occurs in many of mechanical systems. In recent decades, it has been the focus of a number of studies. Investigating the dynamic behavior of mechanical systems often requires modeling contact between two or more components of the system and using detailed finite element models is quite complicated and almost impossible to implement which generally leads to inaccurate predictions. In general, an experimental approach based on the identification of contact parameters appears to be more favorable because of its efficiency.

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Surface roughness has a key role on contact interface attributes and is responsible for nonlinear characteristics of the contact, which can change the dynamic behavior of the system in different vibration amplitudes. The interface is in stick regime at low vibration amplitudes, i.e. the system behaves linearly. As the response level increases, nonlinear mechanisms such as slippage and micro impact start to develop. Research works on contact friction characteristics have a rich history. Den Hartog [1] and Dowell [2] are the pioneers in this field followed by comprehensive investigations of friction phenomenon performed by Ferri [3] and Berger [4]. A large number of models have been developed to simulate the friction effects on mechanical systems. The Iwan model [5] is commonly used to model micro slip. It consists of networks of parallel Jenkin's elements allowing to model partial slip in the contact interface. There are other friction models offering smooth transition from stick to macro slip. The Dahl model [6], the Valanis model [7], the Leuven model [8,9] and LuGre friction model [10] are some examples. Gaul and Lenz [11] performed an experimental study to verify the capability of the Valanis model. Segalman [12] inspected the validity of the Iwan model experimentally. Gaul and Nitsche [13] performed a comprehensive overview on a range of constitutive models for contact interface mechanisms.

A contact model must be capable of taking into account the main nonlinear characteristics involved in the interface. Investigations on the contact dynamics are performed in three fields; the first is studying the friction characteristics in contact interface and only the tangential component of the contact force is considered and normal force assumed to be constant. All the mentioned models [1–13] fall in the first group. The application of these models is limited to contacts with simple geometry and negligible normal motion. In these models the coupling of normal motion and tangential vibration at the interfaces is ignored and the main outcome of this assumption is the symmetry of hysteresis loops. However in experimental observations asymmetric hysteresis loops are frequently seen indicating two returning curves of the hysteresis loops are not following the same trend. These different curves are the result of normal load variations. The second field of study is inspecting the behavior of interface in normal direction and considers a frictionless contact. The application of this approach is limited to collision of multi-body systems or contact with perfectly frictionless surfaces [14,15]. And the third field is developing a general contact model considering the interaction of normal and tangential vibration at the interface. Considering impact and friction in contact interface, Han and Gilmore [16] employed static and kinetic coefficients of friction to relate normal and tangential components of the contact force in their model. A general regularized contact model is developed by Gonthier et al. [17] which include normal compliance, energy dissipation and friction force using seven parameters. However these two models ignore the effect of normal load on tangential stiffness. Gaul and Mayer [18] used finite element approach to introduce an improved method to model contact interfaces. They adopted a nonlinear stiffness to model impact force between the contact surfaces. Yang et al. [19] employed Jenkins element to investigate the effect of normal load on hysteresis loop which only describes the full-slip or full-stick situation. In all published articles simplifications are used and to the authors knowledge there is no proper micro slip model capable of taking into account the effect of normal load variation to generate hysteresis loops observed in micro/macro slip region.

In this paper, a generalized Iwan model is developed. Original Iwan model is capable of reproducing the important contact properties as they are now understood and many authors used this model to predict contact behavior in structures. However the applications of Iwan model are restricted to cases with constant level of normal force. The physical based parametric Iwan model enables one to derive their characteristics directly from experimental data, and this is another reason for employing the model in this investigation. The model is generalized to include the effect of normal load variation in the contact. This is achieved by special scaling of the distribution function and the shear stiffness variations.

The remainder of this paper is organized as follows: in Section 2 the generated force by Iwan model in presence of normal load variation is obtained and properties of derived model are discussed. Sections 3 and 4 describe the mathematical modeling of set-up and the test procedure respectively. In Section 5, Iwan distribution function in high preload condition is identified. To ensure the validation of identified parameters two different identifications approaches based on the energy dissipation function and the force state mapping are employed. In Section 6 by reducing the preload, identified model is used to regenerate the measured data in variable preload.

#### 2. Generalized Iwan contact model

Iwan's model composed of an infinite number of spring-slider arrays, shown in Fig. 1, known as Jenkins elements [5]. Jenkins element is an ideal elasto-plastic element, composed of a single discrete spring in series with a Coulomb damper with a critical slipping force. The Iwan model represents hysteretic features and models transitions in stick–slip states, which appears in a contact. Applied tangential forces to the model, distributes between Jenkins elements and obligate sliders with low critical slipping forces to begin to saturate and slip. This phenomenon known as micro-slip causes softening effect and energy dissipation at the contact interface. Increasing the applied force makes more sliders to slip, finally, at the "ultimate force" all dampers would saturate and the full contact's slip begins. Critical slipping force of frictional sliders  $f^*$  is shown by a distribution density function  $\varphi(f^*)$ , thus  $\varphi(f^*)df^*$  is the fraction of sliders which their critical slipping force is between  $f^*$  and  $f^* + df^*$ .

A typical Iwan model force–displacement hysteresis loop is shown in Fig. 2. The force required for deformation along the path a–b, often referred to as "backbone curve", is:

$$f_{ab}(x)=\int_0^{kx}f^*arphi(f^*)df^*+kx\int_{kx}^\inftyarphi(f^*)df^*,$$

(1)



Fig. 1. Iwan spring-slider model.



Fig. 2. A typical symmetrical hystresis loop.

where k is the spring stiffness and x is the displacement. The "returning curve" b-c-d in Fig. 2 is defined as:

$$f_{bcd}(x,A) = -\int_{0}^{\frac{k(A-x)}{2}} f^{*}\varphi(f^{*})df^{*} + \int_{\frac{k(A-x)}{2}}^{kA} [kx - (kA - f^{*})]\varphi(f^{*})df^{*} + kx\int_{kA}^{\infty}\varphi(f^{*})df^{*},$$
(2)

where *A* is the displacement amplitude and curve d–e–b, also shown in Fig. 2, is reflection of curve b–c–d. It's known the variation of normal load affects parameters of Iwan model. The two parameters of Iwan model are its distribution function,  $\varphi(f^*)$ , and the restoring stiffness, *k*. First we consider the change of the distribution function on generated force.

#### 2.1. Variation effects of distribution function

The sliders in Iwan model are coulomb type and their saturation force is proportional to the normal load. Assuming the distribution function  $\varphi(f^*)$  is obtained at a reference normal load,  $N_{Ref}$ , it is extended to other normal loads, N, by introducing a new parameter as:

$$\alpha(t) = \frac{N(t)}{N_{Ref}},\tag{3}$$

One may use the parameter  $\alpha$  to dilate or compress the reference distribution function as:

$$\hat{\varphi}(f^*,\alpha) = \frac{\varphi(f^*,\alpha)}{\alpha}.$$
(4)

This means the distribution function varies during one cycle and the slider with saturation force of  $f^*$  at reference normal load now may begin to slide at  $\alpha f^*$ .

One needs to find how this variation during one cycle in Iwan model affects the contact restoring force. To answer this question, a differential form is used to find the relation between the change in restoring force and the change of normalized

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 $(d\alpha)$  and displacement (dx) Let

load ratio ( $d\alpha$ ) and displacement (dx). Let us suppose the normal load ratio is increasing and in each step of displacement, the sliders with saturation force less than or equal to kx will slide. In constant preload, by increasing the displacement, the population of saturated Jenkins elements increases as well but situation is different in the presence of variable normal load. As critical force of Jenkins elements are functions of preload, a rapid jump in preload may causes many or all Jenkins elements go to stick state. A criterion for this situation is considering the modified distribution function of Eq. (4) and to determine the change in critical force of the strongest saturated Jenkins element from reference state due to incremental change of contact normal load. These parameters for an element starts to slide at displacement x is:

$$f^{*@x} = kx = \alpha f_{Ref}^{*@x}.$$
(5)

Further increasing the displacement results:

$$f^{*@x+dx} = k(x+dx) = (\alpha+d\alpha) f^{*@x+dx}_{Ref}.$$
(6)

As the reference critical force of the strongest saturated element which goes to slip increases, the population of saturation elements increases and vice versa. At  $d\alpha = 0$ , one finds  $f_{Ref}^{*@x+dx} > f_{Ref}^{*@x+dx}$  and in general cases:

$$d\alpha/\alpha < dx/x \rightarrow f_{Ref}^{*@x+dx} > f_{Ref}^{*@x}.$$
(7)

Physical explanation of this relation is the relative change of restoring force in a particular element (kdx/kx) should be larger than relative change in shear strength of that element  $(d\alpha/\alpha)$ . Satisfaction of this condition increases the percentage of saturated sliders, otherwise if this condition does not meet the sliders which were sliding at former state, go to sticking mode at x + dx. In this situation there would be an element with critical force  $kx_s(x_s < x)$  which satisfies  $d\alpha/dx = \alpha/x_s$ . In physical sense, the sliders with variable saturation force of equal or less than  $kx_s$  remain in sliding mode and other sliders would be at sticking region. The generated force by Jenkins element which is in first group is multiplied by  $d\alpha/\alpha$  and added to kdx in the second group:

$$df = \frac{d\alpha}{\alpha} \int_0^{kx_s} f^* \frac{\varphi(f^*/\alpha)}{\alpha} df^* + k dx \int_{kx_s}^\infty \frac{\varphi(f^*/\alpha)}{\alpha} df^*.$$
(8)

In this article we suppose the condition in Eq. (7) is always satisfied ( $x_s > x$ ). In other words normal load increases gradually. In this situation the strongest slider at current step starts to slide at next step, has saturation force  $k(x + dx)/(1 + d\alpha/\alpha)$  and the differential force is:

$$df = \frac{d\alpha}{\alpha} \int_{0}^{kx} f^* \frac{\varphi(f^*/\alpha)}{\alpha} \varphi_{last}(f^*) df^* + \int_{kx}^{k\frac{x+d\alpha}{1+d\alpha/\alpha}} (f^*(1+d\alpha/\alpha)-kx) \frac{\varphi(f^*/\alpha)}{\alpha} df^* + kdx \int_{k\frac{x+dx}{1+d\alpha/\alpha}}^{\infty} \frac{\varphi(f^*/\alpha)}{\alpha} df^* = \frac{d\alpha}{\alpha} \int_{0}^{kx} f^* \frac{\varphi(f^*/\alpha)}{\alpha} df^* + kdx \int_{kx}^{\infty} \frac{\varphi(f^*/\alpha)}{\alpha} df^*.$$
(9)

Analytical integration of Eq. (9) by a change of variables leads to:

$$f(\mathbf{x}) = \alpha \int_0^{kx/\alpha} f^* \varphi(f^*) df^* + kx \int_{kx/\alpha}^\infty \varphi(f^*) df^*.$$
(10)

On the other hand with decrease of normal load ratio, slipping sliders do not change their state but their slip force declines by the ratio of  $d\alpha/\alpha$  and sliders which change their state from stick to slip have a maximum saturation force of  $k(x + dx)/(1 + d\alpha/\alpha)$ . This leads to the same equation derived for increasing normal load and as a result Eq. (9) can be used for any arbitrary loading pattern and there is no condition or limitation in decreasing pattern.

Eqs. (5)–(10) describe the initial loading pattern of the contact model and form the backbone curve. Next a relation for returning curve is developed using the Masing rule. This rule states that the returning force is composed of the force at returning point and double stretched backbone curve force:

$$f(\mathbf{x}) = f(\mathbf{x}_0) - 2\int_0^{k(\mathbf{x}-\mathbf{x}_0)/2} f^* \varphi(f^*) df^* - k(\mathbf{x}-\mathbf{x}_0) \int_{k(\mathbf{x}-\mathbf{x}_0)/2}^\infty \varphi(f^*) df^*,$$
(11)

where subscript ()<sub>0</sub> denotes the value of parameter in returning point. It is seen the saturation force in the integrals limits equals  $k(x - x_0)/2$ , as part of displacement ( $x_r$ ) returns the springs to the free-state and after that, other part ( $x_t$ ) pulls them in opposite direction:

$$x - x_0 = x_r + x_t. \tag{12}$$

If there was no change in normal load, these two parts would be equal  $(x_r = x_t)$ :

$$x - x_0 = 2x_t \rightarrow x_t = (x - x_0)/2,$$
 (13)

and limits of the integrals would be  $kx_t$ . But when normal load varies one finds:

$$x_t = \frac{\alpha}{\alpha_0} x_r \rightarrow x_t = \frac{x - x_0}{1 + \alpha_0 / \alpha}.$$
(14)

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The differential change of restoring force equals:

$$df = \frac{d\alpha}{\alpha} \int_0^{k(x-x_0)/1+\alpha_0/\alpha} f^* \frac{\varphi(f^*/\alpha)}{\alpha} df^* + k dx \int_{k(x-x_0)/1+\alpha_0/\alpha}^\infty \frac{\varphi(f^*/\alpha)}{\alpha} df^*.$$
(15)

Integrating both sides of Eq. (9) and considering the value of integration at initial point one arrives at:

$$f(\mathbf{x}) = f(\mathbf{x}_0) - (\alpha + \alpha_0) \int_0^{k(x - x_0)/\alpha + \alpha_0} f^* \varphi(f^*) df^* - k(\mathbf{x} - \mathbf{x}_0) \int_{k(x - x_0)/\alpha + \alpha_0}^\infty \varphi(f^*) df^*.$$
(16)

#### 2.2. The effects of stiffness variations on contact restoring force

Considering the influence of stiffness variation, which is a function of normal load itself in Iwan restoring force model  $(k = k(\alpha))$ , one obtains the following condition:

$$|dx/x| \ge |dk/k|. \tag{17}$$

This condition ensures us the reverse sliding doesn't happen. To understand this phenomenon, consider a saturated Jenkins element, shown in Fig. 1, is pulled to the right direction and its slider is drawing in the same direction, now if the direction of displacement changes (passing the returning point) and at the same time, stiffness has a sudden increase, slider slips in the right direction to reduce the elongation of spring and will make the same critical force as before, this slippage is in the opposite direction of the current displacement and is called reverse sliding.

As the stiffness is changed, the backbone curve remains in the original form and no modification is required. However to determine the returning curve from Masing rule some difficulty arises. For instance when the stiffness of particular Jenkins element reduces, the force changes depend on the position of the slider with respect to its reference point. The Masing's form of the element is:

$$f(\mathbf{x}) = f(\mathbf{x}_0) - \beta_1 \int_0^{\beta_2 \mathbf{x}} f^* \varphi(f^*) df^* - k(\mathbf{x} - \mathbf{x}_0) \int_{\beta_2 \mathbf{x}}^{\infty} \varphi(f^*) df^*,$$
(18)

where  $\beta_1$  and  $\beta_2$  are unknown. To obtain these parameters we use the original relation for returning curve as:

$$f(x) = -\int_{0}^{\frac{k(A-x)}{2}} f^{*}\varphi(f^{*})df^{*} + k\int_{\frac{k(A-x)}{2}}^{kA} [x - (A - f^{*}/k)]\varphi(f^{*})df^{*} + kx\int_{kA}^{\infty} \varphi(f^{*})df^{*},$$
(19)

The first integral defines parts of forces generated by saturated elements, second integral represent forces of elements saturated in initial loading and now are at *x* in elastic mode and finally the last integral refers to the elements never be saturated. The limits of integrals are function of *k* which varies in a loading cycle. These limits are defined using the properties of the element at the returning point as follows. In each saturated Jenkins element with constant normal load the elongation of spring is constant ( $x_c = f^*/k$ ). As normal load varies, this elongation changes over the loading path and at the returning point it would be  $f^*/k_0$ . We consider the displacement from returning point as sum of two parts defined in Eq. (12):

$$f^* = k_0 x_r = k x_t \to x_t = \frac{x - x_0}{1 + k/k_0}.$$
(20)

Eq. (20) specifies the upper limit of the first integral and lower limit of second integral as:

$$kx_t = \frac{kk_0(x - x_0)}{k + k_0}.$$
(21)

The upper limit of second integral is  $k_0A$  as long as it is less than  $kx_t$ . Moreover the integrant in second integral must be modified from  $A - f^*/k$ , the elongation of springs at returning point, to  $A - f^*/k_0$ . Applying these modifications, the returning curve relation defined in Eq. (19) is rewritten as:

$$f(\mathbf{x}) = -\int_{0}^{\frac{kk_{0}(A-\mathbf{x})}{k+k_{0}}} f^{*}\varphi(f^{*})df^{*} + k\int_{\frac{kk_{0}(A-\mathbf{x})}{k+k_{0}}}^{k_{0}A} [\mathbf{x} - (A - f^{*}/k_{0})]\varphi(f^{*})df^{*} + kx\int_{k_{0}A}^{\infty}\varphi(f^{*})df^{*}.$$

$$(22)$$

Direct employment of Eq. (22) faces two difficulties:

- (I) One must insure the inequality  $\frac{kk_0(A-x)}{k+k_0} \leq k_0 A$  is satisfied,
- (II) The amplitude A is meaningful in steady state response but not applicable in transient response.

Therefore it is more convenient to calculate the restoring force from Masing rule, and Eq. (22) may be defined in Masing form as:

$$f(x) = \frac{k}{k_0} f(x_0) - \left(1 + \frac{k}{k_0}\right) \int_0^{\frac{kk_0}{k+k_0}(x-x_0)} f^* \varphi(f^*) df^* - k(x-x_0) \int_{\frac{kk_0}{k+k_0}(x-x_0)}^{\infty} \varphi(f^*) df^*.$$
(23)

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Comparing Eqs. (23) and (18), it becomes clear why the original form of Masing rule could not be used and we call the above form the adjusted Masing rule.

#### 2.3. Combined effects of the stiffness and distribution function variations

The final step is to combine these results together and consider both effects of normal load on the distribution function and the stiffness. Rewriting Eq. (16) in original Iwan form:

$$f(\mathbf{x}) = -\int_{0}^{\frac{k(n-\alpha)}{1+\alpha_0/\alpha}} f^* \frac{\varphi(f^*/\alpha)}{\alpha} df^* + k \int_{\frac{k(n-\alpha)}{1+\alpha_0/\alpha}}^{\frac{\alpha}{\alpha}kA} \left[ \mathbf{x} - \left( A - \frac{\alpha_0 f^*}{k\alpha} \right) \right] \frac{\varphi(f^*/\alpha)}{\alpha} df^* + k\mathbf{x} \int_{\frac{\alpha}{\alpha_0}kA}^{\infty} \frac{\varphi(f^*/\alpha)}{\alpha} df^*, \tag{24}$$

and combining it with Eq. (22), leads to:

1.4 ...

$$f(\mathbf{x}) = -\int_{0}^{\frac{k(n+\lambda)}{l+\frac{k\alpha_{0}}{k_{0}\alpha}}} f^{*} \frac{\varphi(f^{*}/\alpha)}{\alpha} df^{*} + k \int_{\frac{k(n+\lambda)}{l+\frac{k\alpha_{0}}{k_{0}\alpha}}}^{\frac{\alpha}{k_{0}k_{0}A}} \left[ \mathbf{x} - \left( \mathbf{A} - \frac{\alpha_{0}f^{*}}{k_{0}\alpha} \right) \right] \frac{\varphi(f^{*}/\alpha)}{\alpha} df^{*} + k x \int_{\frac{\alpha}{z_{0}k_{0}A}}^{\infty} \frac{\varphi(f^{*}/\alpha)}{\alpha} df^{*}.$$

$$(25)$$

Then the final form of adjusted Masing rule can be written as:

$$f(\mathbf{x}) = \frac{k}{k_0} f(\mathbf{x}_0) - \left(\alpha + \frac{k}{k_0} \alpha_0\right) \int_0^{\frac{kk_0}{kx_0 + k_0 \alpha}(\mathbf{x} - \mathbf{x}_0)} f^* \varphi(f^*) df^* - k(\mathbf{x} - \mathbf{x}_0) \int_{\frac{kk_0}{kx_0 + k_0 \alpha}(\mathbf{x} - \mathbf{x}_0)}^{\infty} \varphi(f^*) df^*.$$
(26)

It's worth mentioning based on Eq. (7) limiting criterion for this relation is:

$$\left|\frac{dx}{x}\right| \ge \left|\frac{d\alpha}{\alpha} - \frac{dk}{k}\right|.$$
(27)

However for small change of normal load, provided the variation of stiffness is a linear function of normal load i.e.  $\alpha(t) = \beta k(t)$  where  $\beta = \text{constant}$ , results  $|d\alpha/\alpha - dk/k| = 0$  and criterion (27) is automatically satisfied.

A numerical simulation is provided to show the variation of normal load effects on the hysteresis loops in the contact interferes. An Iwan model with a uniform distribution function spans from 0. to 5. i.e.  $\varphi(f^*) = 0.2(1 - Heaviside(f^* - 5))$  is used to model the contact interface. The model is excited such that it has a cyclic motion in non-dimensional form as  $3 \sin(2\pi t)$ . The restoring force induced in Iwan model versus displacement is shown in Fig. 3 for two distinct set of variables  $k = \alpha = 1 + 0.5 \sin(2\pi t)$  and  $k = \alpha = 1 + 0.5 \cos(2\pi t)$ . Fig. 3 shows how the variation of normal load affects the hysteresis loops in the contact interfere.

#### 3. Modeling a beam with frictional contact support

In this paper, the friction force between two contacting surfaces is investigated when the contact normal force is varied considerably. In order to study this situation, the dynamic behavior of a clamped-frictionally supported beam shown in Fig. 4 is inspected. A normal preload is provided to the contact point by suspended mass blocks. In the presence of a large preload, the lateral movement of the beam support is prevented. Furthermore the variation of normal force due to vibration of the beam is negligible compared to the contact preload and one may suppose that the contact normal force is constant. In the present study first the preload is set large enough and the Iwan parameters in presence of constant preload are identified. Next, the preload is reduced such that its variation in the contact cannot be disregarded. In this situation the contact normal



**Fig. 3.** Effect of preload on hysteresis loops. (+) constant preload and (o) variable normal load,  $k = \alpha = 1 + 0.5 \sin(2\pi t)$  (left) and  $k = \alpha = 1 + 0.5 \cos(2\pi t)$  (right).

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Fig. 4. The schematic view of beam.

force is not known and varies periodically due to harmonic external excitations. The contact normal force variations needs to be considered in the modeling of the system and must be identified to predict the tangential contact force.

Euler–Bernoulli beam theory is employed to model the dynamic response of the beam. The beam has a modulus of elasticity of *E*, cross sectional moment of inertia of *I*, mass density of  $\rho$ , cross sectional area of  $\overline{A}$ , and length of *L*. In the model axial inertial effects of beam are neglected as the dynamic behavior is considered in a frequency range much lower than its first axial mode. Therefore, in axial direction the beam is regarded as a spring of stiffness  $k_b$  which is equal to  $E\overline{A}/L$ . The frictional support is composed of a pin welded to the right end of the beam and is allowed to slip on a steel block. The pin has a radius of *r*, mass of  $m_p$ , mass moment of inertia of  $J_p$ . Applying a normal force to the rod provides a preload to the contact interface. This is achieved, as shown in Fig. 5, by suspended mass blocks attached to the rod via a string. The beam is excited by concentrated force *F*(*t*) applied at distance *S* from clamped end.

The beam equation of motion is:

$$El\frac{\partial^4 w}{\partial x^4} - F_{nl}(t)\frac{\partial^2 w}{\partial x^2} + \rho \bar{A}\frac{\partial^2 w}{\partial t^2} = F(t)\delta(x-S) - rF_{nl}(t)\delta'(x-L).$$
(28)

where  $F_n(t)$  is the non-linear friction force at the contact. The normal stiffness of the contact interface is proportional to the contact preload. It is assumed, within the range of preload variations in the current study, this relation is linear and is represented with stiffness  $k_n$  located between two contact surfaces. The beam is clamped at one side and loaded with shear force at the other side. The shear force at beam frictional support can now be established using three terms, a static preload equal to total weight of suspended masses, preload variation caused by relative lateral movement of surfaces and inertia force of block masses,

$$EI\frac{\partial^3 w(L,t)}{\partial x^3} = k_n w(L,t) + n_b m \left(\frac{\partial^2 w(L,t)}{\partial t^2} - g\right),\tag{29}$$

where  $n_b$  is the number of blocks and m = 7 kg is the mass of each of them.

One may solve Eq. (28) with the given boundary conditions to obtain the non-linear friction force  $F_{nl}(t)$ . The friction force and the shear deformation at the contact interface are needed to define hysteresis loops of the contact interface. The contact shear deformation is governed by three different effects included in the right hand side of the following identity:

$$u(t) = -\frac{1}{2} \int_0^L \left(\frac{\partial w(x,t)}{\partial x}\right)^2 dx + r \frac{\partial w(L,t)}{\partial x} + \frac{F_{nl}(t)L}{\bar{A}E}.$$
(30)



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The first effect is related to shortening of the beam due to its lateral bending motion and is described by the first term on the right hand side of Eq. (30). The second term is the relative motion due to rotation of the beam end, and last term indicates the axial deformation of beam due to friction force at the contact interface.

To reduce the order of the nonlinear model of Eq. (28), the Galerkin method is employed. The friction in contact is displacement dependent phenomenon and corresponding base linear system mode shapes are function of amplitude. The nonlinear response of the beam is expanded using the mode shapes of the base linear system as:

$$w(x,t) = \sum_{i=1}^{n} \tilde{\phi}_i(x,a) q_i(t), \tag{31}$$

where a is the amplitude of response at direct measurement point. Employing this expansion series in Eq. (28) and using their orthogonally properties, the discretized nonlinear equations of beam motion are:

$$\begin{split} \ddot{q}_{i}(t) + \omega_{i}^{2}(a)q_{i}(t) - F(t)\tilde{\phi}_{i}(S,a) - k_{\theta}(a) \left(\sum_{r=1}^{n} q_{r}(t) \frac{\partial \tilde{\phi}_{r}(L,a)}{\partial x}\right) \frac{\partial \tilde{\phi}_{i}(L,a)}{\partial x} \\ &= \left(r \frac{d \tilde{\phi}_{i}(L,a)}{dx} - \sum_{r=1}^{n} q_{r}(t) \int_{0}^{L} \frac{\partial \tilde{\phi}_{r}(x,a)}{\partial x} \frac{\partial \tilde{\phi}_{i}(x,a)}{\partial x} dx\right) F_{nl}(t) - \tilde{\phi}_{i}(L,a)(k_{n}w(L,t) - n_{b}mg), \quad i = 1, 2, \dots, n. \end{split}$$
(32)

where  $k_{\theta}(a)$  is the support equivalent flexural stiffness at different vibration levels. In the following section, experiments on the structure shown in Fig. 4 are performed. The observed behavior of test structure is used to determine the mode shapes of the base linear system, the frictional and normal contact forces.

#### 4. Experimental case study

The experimental case is a steel beam clamped at one end and fixed at other end with frictionally support as shown in Fig. 5. The dimension of beam are L = 600 mm (length), b = 40 mm (width) and h = 5 mm (thickness). The rod attached to the end of beam end is steel and it has a radius of r = 5 mm and has the length the same as the beam width. The weight of suspended mass block provides the desired value for preload and this allows for the application of arbitrary preloads on the contact interface. A *B&K4200* mini shaker is used to excite the beam through a stinger at distance S = 550 mm from the clamped side. A *B&K8200* force transducer is located between the beam and stinger to measure the excitations. Three accelerometers *AJB* 120 are mounted on structure to measure the lateral acceleration and placed at distance  $x_1 = 550 \text{ mm}$ ,  $x_2 = 300 \text{ mm}$  and  $x_3 = 50 \text{ mm}$  from clamped side. A laser Doppler *OMETRON vh-1000-d* is employed to measure axial deformation of beam at frictional end.

Two parameters of Iwan model, namely the stiffness of Jenkins elements, i.e. the stiffness of contact at stick state, and the distribution function must be identified. To identify the stiffness at the stick state, the beam is excited with a low amplitude random signal and frequency response function (FRF) is measured. The FRFs of the beam for two different preloads is shown in Fig. 6. In the mathematical model, the stiffness at contact is tuned in such a way to regenerate the same resonance frequencies.

The identification of distribution function is more complicated. There are a few approaches for extracting this function. Song et al. [20] and Shiryayaev et al. [21] applied neural network method to identify the parameter of a uniform distribution function from time domain response. Segalman [22,23] proposed the distribution function with four unknown parameters. Ahmadian et al. [24] applied the force state mapping method to identify the hysteresis loop on contact interfere which could be used to identify the Iwan parameters. It is shown in the previous work of the authors that the distribution function could be identified by dissipation energy pattern in different displacement amplitude [25].



Fig. 6. Linear frequency response curve for direct measurement.

All of these methods are restricted to the cases with constant preload. To provide this condition, we used three mass blocks (21 kg) as the preload and two later approaches is employed at the same time to validate the identified distribution function.

#### 5. Distribution function identification in a high preload

In test strategies of nonlinear systems it is common to excite the structure near one of its resonances using a single harmonic force. This practice leads to a dominant single mode response in systems without modal interaction and the contribution of other modes is usually marginal.

The measured force and acceleration signals are used to reconstruct the nonlinear restoring forces and shear deformation in the contact interface. The first part of this section deals with determination of the shape functions used in Galerkin projection. Then the measured accelerations and identified shape functions are employed to calculate the generalized coordinates using Eq. (31).

The response of the structure contains one dominant harmonics close to the first natural frequencies of the system. Therefore, the mode shapes of the base linear system are good approximations for series expansion base functions. As mentioned in previous section, when the structure behaves linearly the contact interface is modeled by tangential spring. Eliminating nonlinear parts of Eq. (28) and replacing the last term with  $k_{\theta}$  leads to a linear equation of motion. This stiffness identified from test result and selected in such a way to regenerate resonance frequencies observed in the test.

It should be noted in Eq. (28) that the effect of accelerometers is not introduced for simple understanding but they are considered in the model. Using the section at place of each accelerometer, the beam divides into four parts and compatibility requirements at the interface of each two parts are added to the equations of motion. It is assumed that the displacements and slopes at the sections don't change but the shear forces and bending moments alter due to mass and inertia of the accelerometers and the force transducer. Now, the linear base system is completely described. Having the linear mode shape, the generalized coordinate vector can be calculated using the measured accelerations at 3 points:

$$\ddot{q}(t) = \begin{cases} \ddot{q}_{1}(t) \\ \ddot{q}_{2}(t) \\ \ddot{q}_{3}(t) \end{cases} = \begin{bmatrix} \tilde{\phi}_{1}(x_{1},a), \tilde{\phi}_{2}(x_{1},a), \tilde{\phi}_{3}(x_{1},a) \\ \tilde{\phi}_{1}(x_{2},a), \tilde{\phi}_{2}(x_{2},a), \tilde{\phi}_{3}(x_{2},a) \\ \tilde{\phi}_{1}(x_{3},a), \tilde{\phi}_{2}(x_{3},a), \tilde{\phi}_{3}(x_{3},a) \end{bmatrix}^{-1} \begin{cases} \ddot{w}(x_{1},t) \\ \ddot{w}(x_{2},t) \\ \ddot{w}(x_{3},t) \end{cases} = \Phi^{-1} \ddot{w}.$$
(33)

Next we turn our attention to the experimental results. The mode shapes of the base linear system are inherently good approximation. Therefore, we donot expect a large number of modes contributing to construct the desired shape functions and the number of mode shapes in Eq. (31) is set to be three. In other words, three linear mode shapes used to identify one shape functions. The corresponding generalized coordinates q(t) of this three modes is shown in Fig. 7. As it is seen in the third linear mode has a marginal contribution in the final shape functions that ensures that there is no need for more modes in construction of the desired shape functions.

The normal stiffness  $k_n$  is the last parameter to be evaluated. The response of structure is insensitive to this parameter so it cannot be calculated by any identification method accurately. Johnson [26] showed that force–displacement for an infinite cylinder on a plane in normal direction would be:

$$\delta = P \frac{1 - v^2}{\pi E} (2 \ln(4r/a_1) - 1), \quad a_1^2 = 4Pr/\pi E^*, \tag{34}$$



**Fig. 7.** A generalized coordinate at 30 m/s<sup>2</sup> for direct measurement.

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where *P* is the compressive load per unit, *E* and  $E^*$  are elasticity modulus and elasticity composite modulus, respectively, *r* is the radius of cylinder and v is the poison ratio. This relation plotted in Fig. 8 for dimension of the cylindrical rod which attached to the beam end. As it is shown a linear spring gives good estimate for this relation.

However infinite length of cylinder and frictionless of surfaces are two assumptions in Eq. (34) and calculated value for stiffness are rough but we use it in our model. That is because as mentioned above structure response is insensitive to this parameter which means Elw''(L,t) or beam support shear force remains invariant. It is shown in Eq. (29) shear force at beam end is summation of three terms which the effect of inertia force of mass blocks is negligible, this leads to the conclusion that the generated force by variation of normal spring remains the same and if spring stiffness decreases a few percent, the displacement amplitude increases in such way to make up this change in generated force. In other words when the normal stiffness exceeds from the certain value, the boundary condition at beam end becomes close to the classical simply support boundary condition (rotation resistance due to friction exists anyway) and changing stiffness has no effect on support reactions.

Another way to identify distribution function is by using contact dissipation energy [25]. It is shown that each distribution function causes a unique pattern of dissipation energy in different amplitude and the relation is as follows:

$$\frac{d^2D}{dA^2} = 4Ak^2\varphi(kA),\tag{35}$$

where *D* is the dissipation energy. Increasing the number of recorded points increases the accuracy of identified function. This approach is less sensitive to noise than force state mapping method. Because in the later one, the nonlinear force is differentiation of two bigger terms (stiffness and inertia force) and noisy data leads to erroneous result.

In this paper, the single sinusoidal excitation is applied to beam at 80 different amplitudes to generate acceleration with amplitude from 40 mg to 6 g at direct point of measurement. In each level, the frequency of excitation is set to the resonance frequency of structure on that level (with overall error 0.1 Hz). This frequency decreases from 57.3 Hz at 40 mg to 54.8 Hz at 6 g. By having contact displacement and contact dissipation energy and using Eq. (35), the distribution function could be extracted.

In Fig. 9 the distribution function identified by dissipation energy method is shown and hysteresis loop in contact for three different amplitude levels by both methods is shown in Fig. 10. These diagrams have a good agreement at amplitude levels up to 3 g after that normal load variation makes diagram to deviate from each other. Therefore if we seek for a reliable distribution function, we could only take some part of this function which normal load effects doesnot contaminate it. In 3 g amplitude level a critical force of the strongest Jenkins element would be:

$$f^* = A_{\text{joint}} \times k = 18 \times 10^{-6} \times 11.02 \times 10^{+6} \simeq 200 \text{ N.}$$
(36)

As it is shown, the deviation between two diagram starts at hysteresis loop end in 3 g level so we could conclude that the distribution function are reliable and almost exact until 200 N on critical force axis.

In dissipation energy method The distribution function extracted on constant normal load assumption because the effect of this variation on dissipation energy is not considered therefore the hysteresis loops are plotted under this condition and variation of normal load neglected but it's worth to mention, if we accept the identified function as an exact one in whole span and taking into the account the influence of normal on generated hysteresis loop, it could be seen that a better agreement between two methods would achieve (Fig. 11) although some deviations remain but it can capture some features.

#### 6. Application of generalized Iwan model

In this section the preload of beam under the test reduces to one mass block and the identified distribution function is applied to regenerate the time domain response of structure in high variation of normal load. The high variation of preload



Fig. 8. Force versus displacement in normal direction. r = 5 mm, E = 210 Mpa, v = 0.3, L = 0.05 m The slope of fitted line is  $k_n = 2.72 \times 10^9$  kN/mm.



Fig. 9. Distribution function of Iwan model extracted by dissipation energy method.



Fig. 10. The hysteresis for three measurement levels 1 g, 3 g and 6 g: the dissipation energy method (solid line) and the force state mapping method (dash line).



Fig. 11. The hysteresis loop in measurement levels 6 g: the dissipation energy method (solid line) and the force state mapping method (dash line).

causes large change in tangential (or equivalent flexural) stiffness in one period of movement so the a few mode shape of base linear system could not reveal the response of structure and it makes the both force state mapping and dissipation energy methods inapplicable. But the physical parameters of Iwan model permit to employ the identified distribution function to evaluate the response.

First in identified distribution function we divide and multiply the horizontal and vertical axis, respectively by 3 to obtain new function in one mass block preload, next the effect of normal load on tangential stiffness should be evaluated. The frequency response of the beam is obtained in three different preloads. The first resonance frequencies and identified tangential stiffness's is shown in Table 1. Fig. 12 shows the variation of this stiffness versus the normalized preload. This parameter variation in one mass block case, is about 50% so to get smooth change, the second degree polynomial is fitted on equilibrium and only two first point of measurement.

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#### Table 1

The effect of preload on the contact tangential stiffness.

| Number of mass blocks                         | One mass block   | Two mass blocks                         | Three mass blocks  |
|---|--|---|--|
| Fundamental frequency<br>Tangential stiffness | $\begin{array}{l} 55.3 \text{ Hz} \\ 4.90 \times 10^6 \text{ N/m} \end{array}$ | 56.52 Hz $8.71 \times 10^6 \text{ N/m}$ | $\begin{array}{l} \text{57.3 Hz} \\ \text{11.02}\times 10^6 \text{ N/m} \end{array}$ |



Fig. 12. The identified tangential stiffness for three points (circles) and fitted curve to estimate curve between two first points (solid line).



Fig. 13. The model generated hysteresis loop.



Fig. 14. Time domain for direct measurement: test data (solid line) and model prediction (dash line).

The generated hysteresis loop is shown in Fig. 13 and to validate the model, the time domain response in direct point of measurement shown in Fig. 14.

By increasing the amplitude, the test result and model prediction diverge from each other. There are two main causes for it, first, the normal spring which is used to model normal load variation is acceptable in limited band and increasing the

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amplitude activates the phenomenon like micro slap and its strong nonlinear effects influence on beam response. Second cause is the domain of reliability of distribution function. As we mentioned the original identified function is reliable until  $18 \times 10^{-6}m$  for tangential displacement amplitude which leads to Jenkins elements with critical force up to 200 N. When the preload reduces, the critical force of this Jenkins elements decrease to one-third of original value but the stiffness of tangential spring reduces too thus we could conclude the acceptable amplitude for contact displacement would be about:

$$A_{\text{joint}}^{\text{one block}} = A_{\text{joint}}^{\text{three blocks}} k_{\text{three blocks}} / 3k_{\text{one block}}.$$
(37)

In our experiment, the reliable amplitude in one mass block preload is about 0.76 of amplitude in three mass blocks.

#### 7. Conclusion

Iwan model is a commonly employed mathematical representation of contact interface behavior. This model is generalized for cases where the variation of normal load has a significant effect on restoring force of contact. The generalized model is capable of producing hysteresis loops observed in experimental data. The generalized model is used to predict the response of a beam with frictional support. The main parameter of Iwan, the distribution function, is identified in constant preload then the other parameter, the shear stiffness, is identified experimentally as a function of the preload. These two parameters are employed to regenerate the structure response in variable preload state. Based on observed results, variable preload affects the obtained hysteresis loops, which in turn is responsible for nonlinear behaviors of the beam.

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